



## EXPERIMENTAL STUDY ON STABILIZATION OF COLLAPSIBLE SOILS USING BENTONITE

1K. CHANDRA MOHAN , 2DASARI SHIVANI

1. M. Tech Student Department of Geotechnical engineering, JNTUH college of engineering,  
(Telangana) .India..

Email-: [kakechandu@gmail.com](mailto:kakechandu@gmail.com)

2.M.Tech Student Department of Geotechnical engineering, JNTUH college of engineering,  
(Telangana) .India.

Email-: [dasarishivani98@gmail.com](mailto:dasarishivani98@gmail.com)

**Abstract:** Folding soils are unsaturated soils that breakdown unexpectedly when wetted under stacking. Greater part of normally happening folding soils are Aeolian stores like loess. Red soils are likewise gone under this classification. A few strategies have been utilized to concentrate on the way of behaving of folding soils by understanding their geotechnical properties. In present review bentonite was picked as an added substance and different geotechnical tests have directed on Ben tonite - Red soil blends. To concentrate because of fines on folding way of behaving, the exploratory work says that the foldability was found to diminishes with expansion of 2, 4, 6,8,10 level of bentonite. The breakdown conduct was evaluated from measures like Gibbs (1961), Denisov (1951) and so on, in light of Geotechnical properties.

**KEYWORDS:** bentonite, Collapsible behavior , Red soils

### 1.INTRODUCTION

Vishakhapatnam region is a rapid industrial urban growth area inhabited by number of infrastructure projects related to roads, residential buildings, embankments, and retaining walls etc., which are found on red soil deposits. Structures founded on these soils subjected to large deformations due to reduction in volume and loss of shear strength when in contact with water causes distress to the structures due to large differential settlements. In the present study red soils were collected and tested for index properties such as grain size distribution, Atterberg's limits, specific gravity and engineering properties like compaction and Strength tests, have been performed.. The degree of collapsible potential is estimated by using index and engineering properties such as grain size distribution, Atterberg's

limits, void ratio, degree of saturation, water contents, compacted densities etc. Gibbs (1961), Prikloński (1952), Fedá (1964), Denisov (1951) Mitchell & Soga (2005), Pereira (2000), Holden Hiff (1961), Rogers(1994), Clemense & Finbar (1981), Latun (1992) etc are extensively studied on collapsible soils.

### II. MATERIALS

The materials used in this investigation are:

1. Red Soils
2. Bentonite

The soil are collected from different regions in Visakhapatnam region, the soil samples were collected at a depth of 1.0 – 1.5m from the ground level and the collected samples were dried and subjected for geotechnical characteristics such as grain size distribution, plasticity, compaction and Shear strength. Bentonite

soil is a type of clayey soil having montmorillonite clay mineral, which expands when, comes in contact with water and shrinks when the water evaporates. it usually forms from weathering of volcanic ash. It is hugely available in North West of India. i.e., Kutch area. The dried bentonite was subjected for geotechnical characterization such as gradation, consistency, swelling etc., as per IS 2720

### III. METHODOLOGY

Exciting methods of Estimation of Collapsible Potential: DENISOV (1951) used coefficient of subsidence (K) which is

a ratio of void ratio at liquid limit to natural void ratio expressed as follows:

Coefficient of subsidence  $K = \text{void ratio at liquid limit} / \text{natural void ratio} (eL/en)$

If,  $K = 0.5 - 0.75$ : highly collapsible soil;

If,  $K = 0.75 - 1.5$  likely to collapsible;

If,  $K = 1.5 - 2.0$ : non collapsible soil GIBBS (1961) proposed a method to measure collapse potential named as in terms as collapsible ratio (R) .

It is defined as Collapsible ratio  $R = w_{sat} / W_l$  R is greater than 1 Collapsible R is Less than 1 Free from collapsible.

## IV RESULTS AND DISCUSSION

### A. GEOTECHNICAL PROPERTIES OF RED SOIL OF VISAKHAPATNAM REGION

Sand (%)	85
Silt (%)	13
Clay (%)	2
Specific Gravity G	2.65
Liquid Limit (%)	21.00
Plastic Limit (%)	18.00
Plasticity Index (Ip)	3.00
IS Classification	<b>SM</b>
(OMC %)	9.20
(MDD g/cc)	1.74
C (t/m <sup>2</sup> )	1.00
Φ (Degrees)	30

### B. GEOTECHNICAL PROPERTIES OF BENTONITE:

<75µm (%)	100
<2µm (%)	78
<1µm(%)	60
Specific gravity	2.71
Liquid limit	486
Plastic limit	72

Plasticity index	414
Is classification	CH
OMC (%)	50
MDD (g/c.c)	1.25
Free swell index (%)	780
Swell pressure (kN/m <sup>2</sup> )	520

### C.VARIATION OF INDEX PROPERTIES OF RED SOIL (SM) WITH BENTONITE :

Bentonite Percentage +SM SOLS	WL( %)	Wp( %)	IP( %)	OMC( %)	MDD (%)
SM+0	21	18	3	9.2	1.74
SM+2%	25	19	6	10.2	1.76
SM+4%	30	20	10	12	1.79
SM+6%	36	21	15	14	1.81
SM+8%	43	22	21	16.5	1.77
SM+10%	50	23	27	19.0	1.72

Test results of Red soil (SM) mixed with Bentonite has the following identifications. Increases the percentage of Bentonite increases liquid limit, plastic limit and plasticity index values. The percentage of Bentonite means increasing the percentage of fines particularly clay particles. These particles require more water to nullify their electric charges which increases a thin film of moisture coated around the particles inform of diffusion is known as diffused double layer. Increasing the percentage of Bentonite increases OMC gradually. This is due to the fines of Bentonite particles. In case of maximum dry density it is identified that increasing the percentage of Bentonite increases dry density by locking of moisture with clay surface upto 6% and then decreases

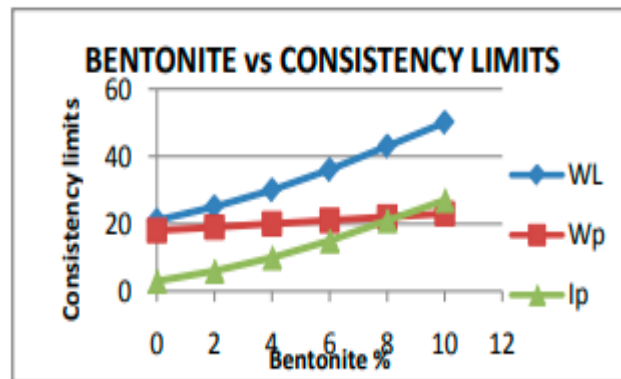
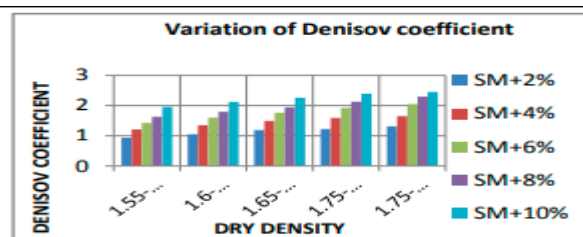


Fig (a)

D.Variation of Denisov coefficient with dry density variation of percentage of bentonite:

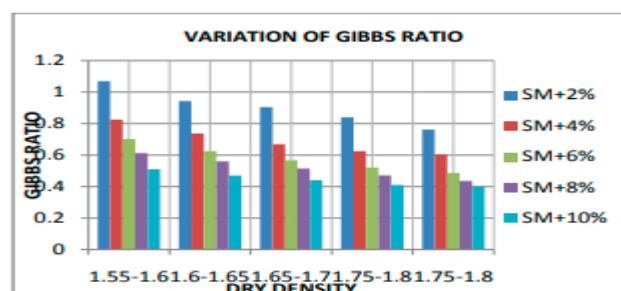
$\gamma_d$ (g/cc)	1.55- 1.6	1.6- 1.65	1.65- 1.7	1.7- 1.75	1.75- 1.8
Soils ↓	Denisov coefficient of subsidence ↓				
SM	0.78	0.85	1.00	1.08	
SM+2%	0.934	1.05	1.186	1.224	1.31
SM+4%	1.211	1.35	1.494	1.59	1.65
SM+6%	1.424	1.6	1.763	1.92	2.05
SM+8%	1.63	1.795	1.942	2.13	2.294
SM+10%	1.957	2.12	2.26	2.39	2.45

$\gamma_d$ (g/cc) ↓	1.55- 1.6	1.6- 1.65	1.65- 1.7	1.7- 1.75	1.75- 1.8
Soils ↓	Gibbs collapsible				
SM	1.32	1.19	1.00	0.93	
SM+2%	1.07	0.943	0.905	0.84	0.762
SM+4%	0.825	0.737	0.669	0.625	0.604
SM+6%	0.702	0.625	0.567	0.521	0.486
SM+8%	0.612	0.56	0.515	0.471	0.436
SM+10%	0.51	0.47	0.44	0.41	0.4



Fig(b)

E.VARIATION OF GIBBS RATIO WITH DRY DENSITY AT DIFFERENT PERCENTAGE OF BENTONITE:



Fig(c)



#### IV. CONCLUSION:

- 1.Red soils compacted at dry densities in between 1.4g/cc-1.5g/cc susceptible to high collapsible 1.5-1.7 and 1.7-1.8g/cc, which are at their MDD are free from collapsible.
- 2.Addition of Bentoniteincreases plasticity index and degree of saturation of SM soils.
- 3.Addition of Bentonite decreases potentials of collapsible behavior when compacted at dry densities in between 1.5g/cc-1.6g/cc.
- 4.Denisov's and Gibbs approaches shows that addition of bentonite to red soils decreasescolaplsiable potential upto 10%

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